

Geometric Design



Outline

- 1. Concepts**
- 2. Horizontal Alignment**
 - a. Fundamentals**
 - b. Superelevation**
- 3. Setting out of Horizontal Curve**

Concepts

- **Alignment is a 3D problem broken down into two 2D problems**
 - Horizontal Alignment (plan view)
 - Vertical Alignment (profile view)
- **Stationing**
 - Along horizontal alignment
 - $12+500 = 12500$ m.

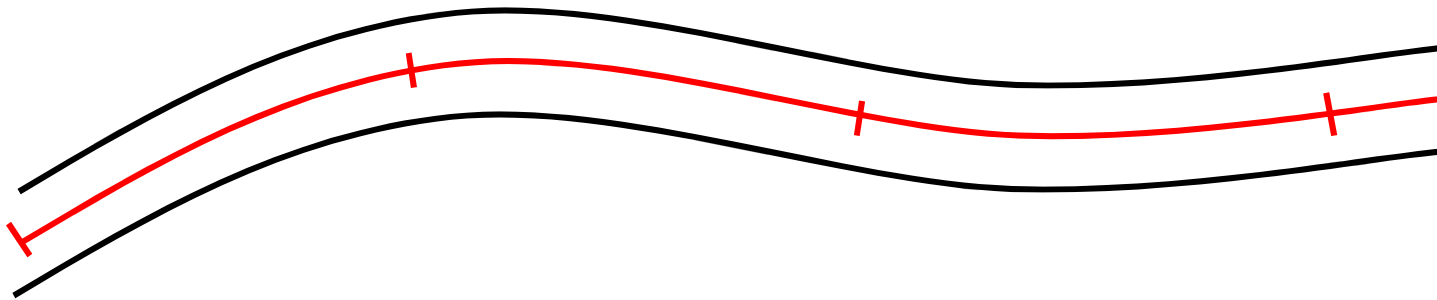


<http://go.to/funpic>

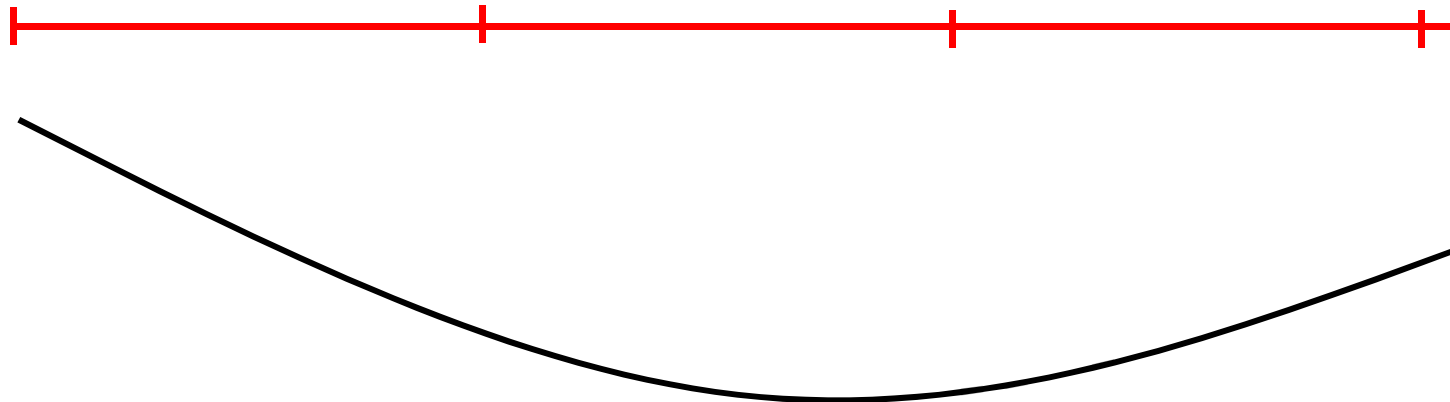
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Stationing

Horizontal Alignment



Vertical Alignment

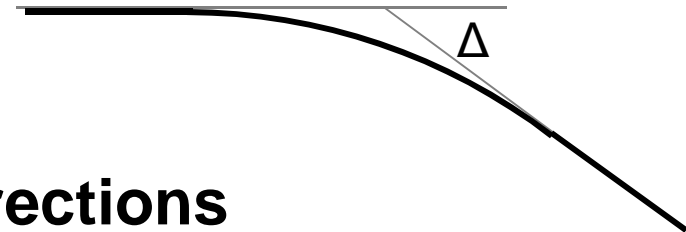


A photograph of a paved road with double yellow lines curving through a forest. The road is dark asphalt and the lines are bright yellow. The forest is dense with green trees and some autumn-colored leaves on the ground. Sunlight filters through the trees, creating dappled light on the road. The text "Horizontal Alignment" is overlaid in the lower-left quadrant of the image.

Horizontal Alignment

Horizontal Alignment

- **Objective:**
 - Geometry of directional transition to ensure:
 - Safety
 - Comfort
- **Primary challenge**
 - Transition between two directions
 - Horizontal curves
- **Fundamentals**
 - Circular curves
 - Superelevation

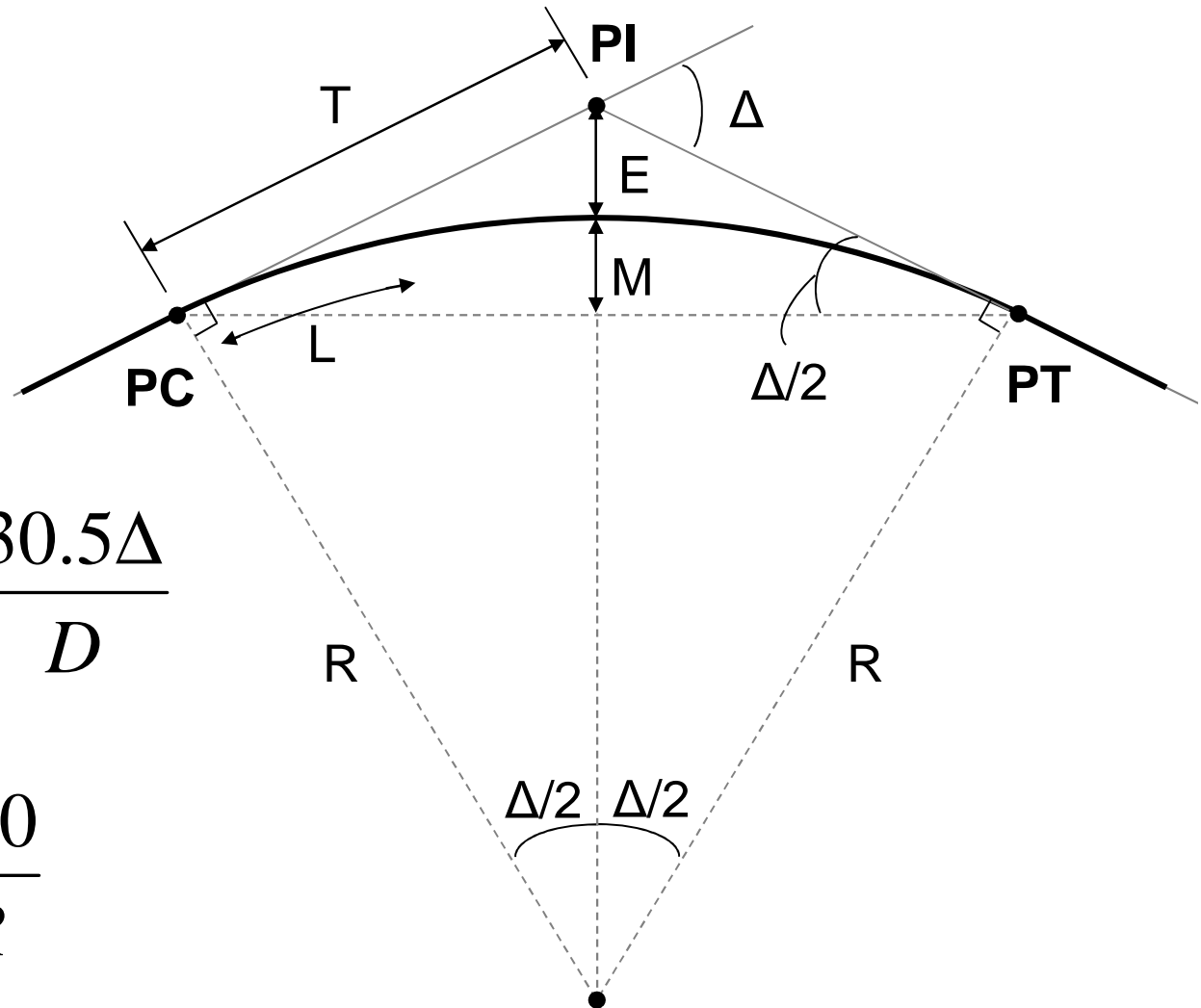


Horizontal Curve Fundamentals

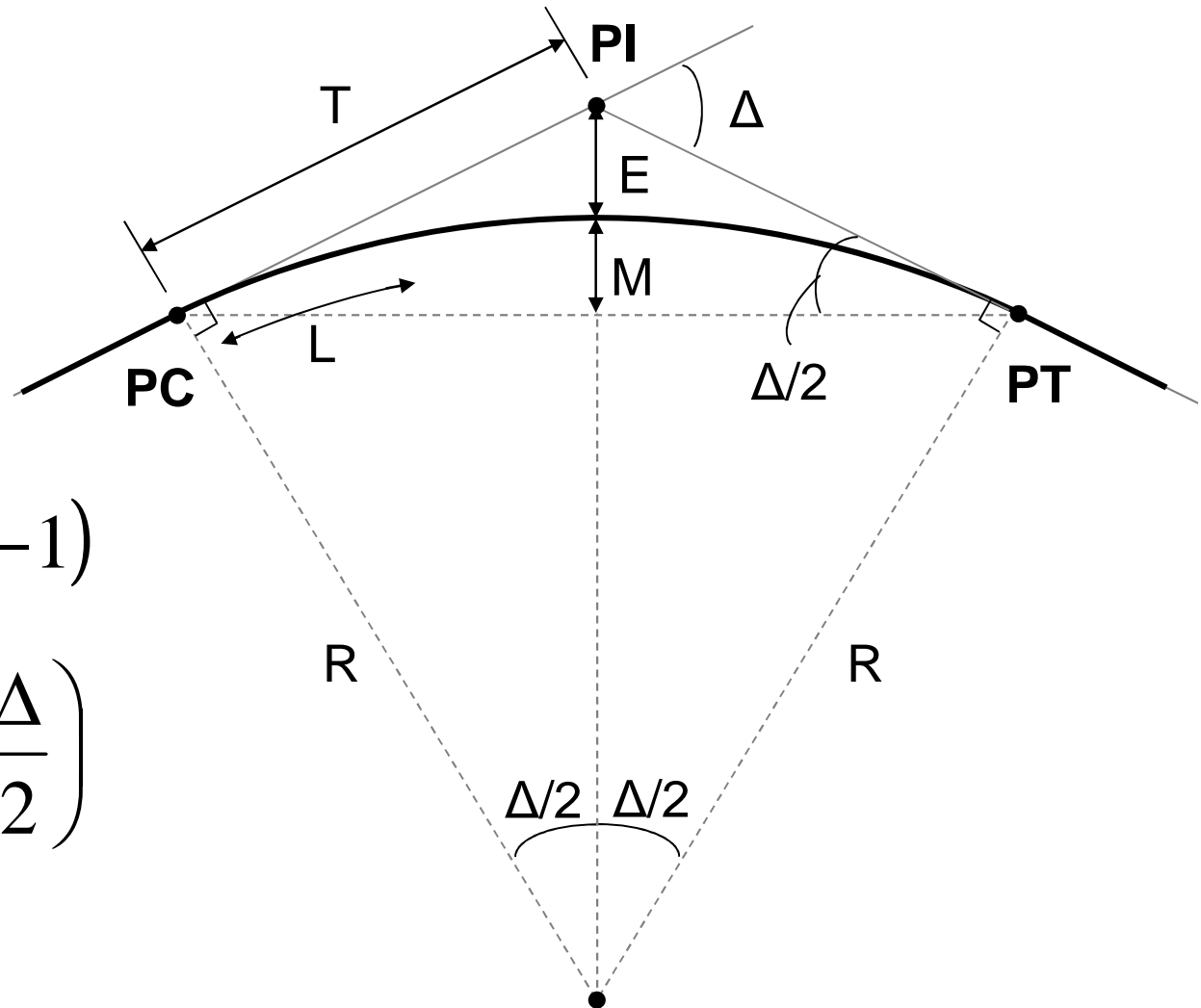
$$T = R \tan \frac{\Delta}{2}$$

$$L = \frac{\pi}{180} R \Delta = \frac{30.5 \Delta}{D}$$

$$D = \frac{1750}{R}$$



Horizontal Curve Fundamentals



$$E = R(\sec \Delta / 2 - 1)$$

$$M = R \left(1 - \cos \frac{\Delta}{2} \right)$$

Example 1

A horizontal curve is designed with a 450 m radius. The tangent length is 120m and the PC station is 2+000. What are the PI and PT stations?

Design Concept

Two Basic Problems that determine the minimum radius of any horizontal curve.

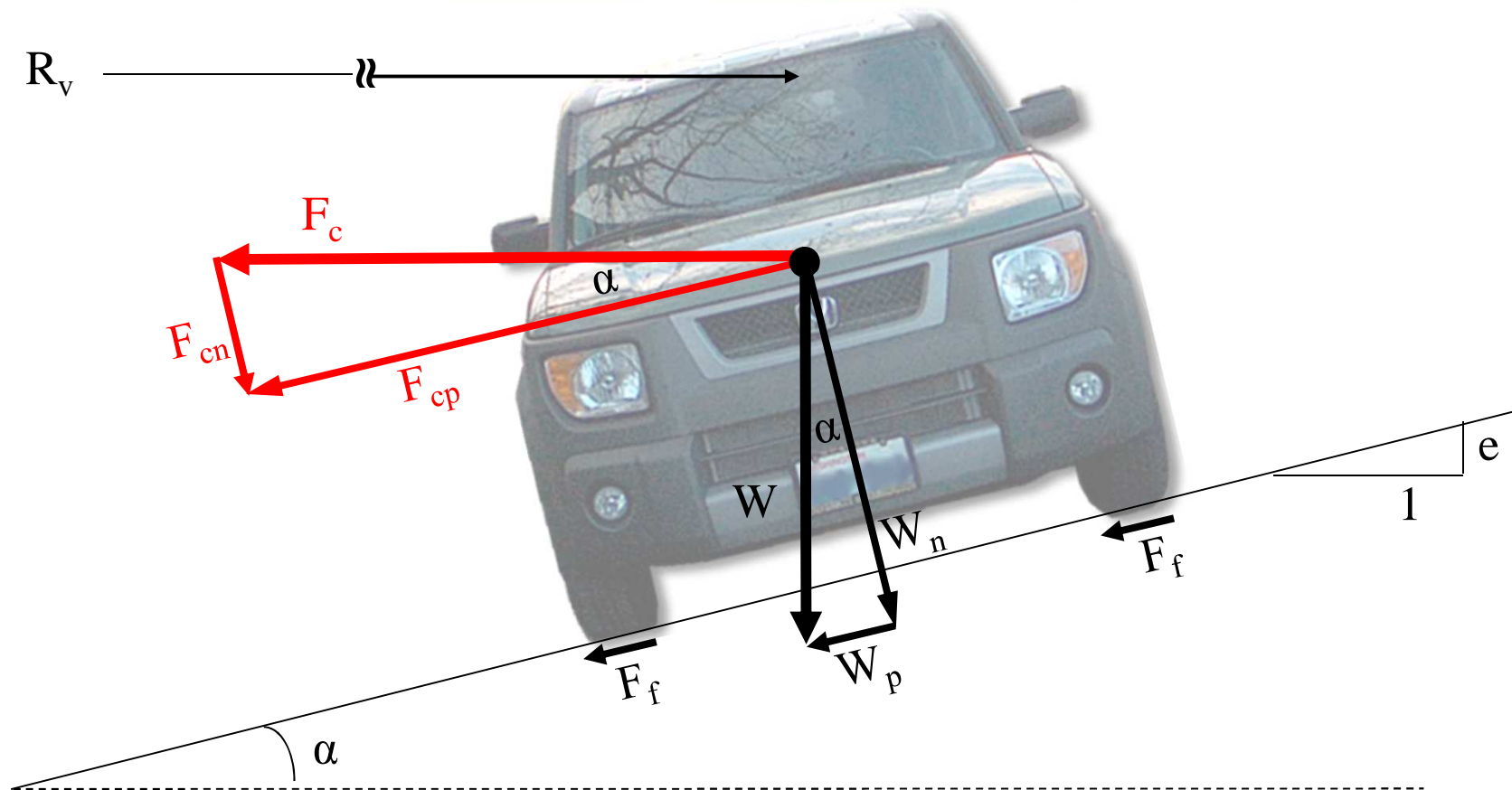
- Balance of vehicle , and
- Sight Distance .



1. Balance of vehicle (Superelevation)

Superelevation

$$W_p + F_f = F_{cp}$$



$$W \sin \alpha + f_s \left(W \cos \alpha + \frac{WV^2}{gR_v} \sin \alpha \right) = \frac{WV^2}{gR_v} \cos \alpha$$

Centripetal or Centrifugal?

- **As a vehicle moves in a circular path**
 - Centripetal acceleration acts on the vehicle in the direction of the center of the curve
- **The acceleration is sustained by**
 - Component of the vehicle's weight related to the roadway superelevation
 - Side friction developed between the vehicle's tires and the pavement surface
 - Or a combination of the two

Superelevation

$$W \sin \alpha + f_s \left(W \cos \alpha + \frac{WV^2}{gR_v} \sin \alpha \right) = \frac{WV^2}{gR_v} \cos \alpha$$

$$\tan \alpha + f_s = \frac{V^2}{gR_v} (1 - f_s \tan \alpha)$$

$$e + f_s = \frac{V^2}{gR_v} (1 - f_s e)$$

$$e + f_s = \frac{V^2}{127R}$$

Centrifugal Force

- **Imaginary force that drivers believe is pushing them outward while maneuvering a curve**
- **In fact, the force they feel is the vehicle being accelerated inward towards the center of the curve**

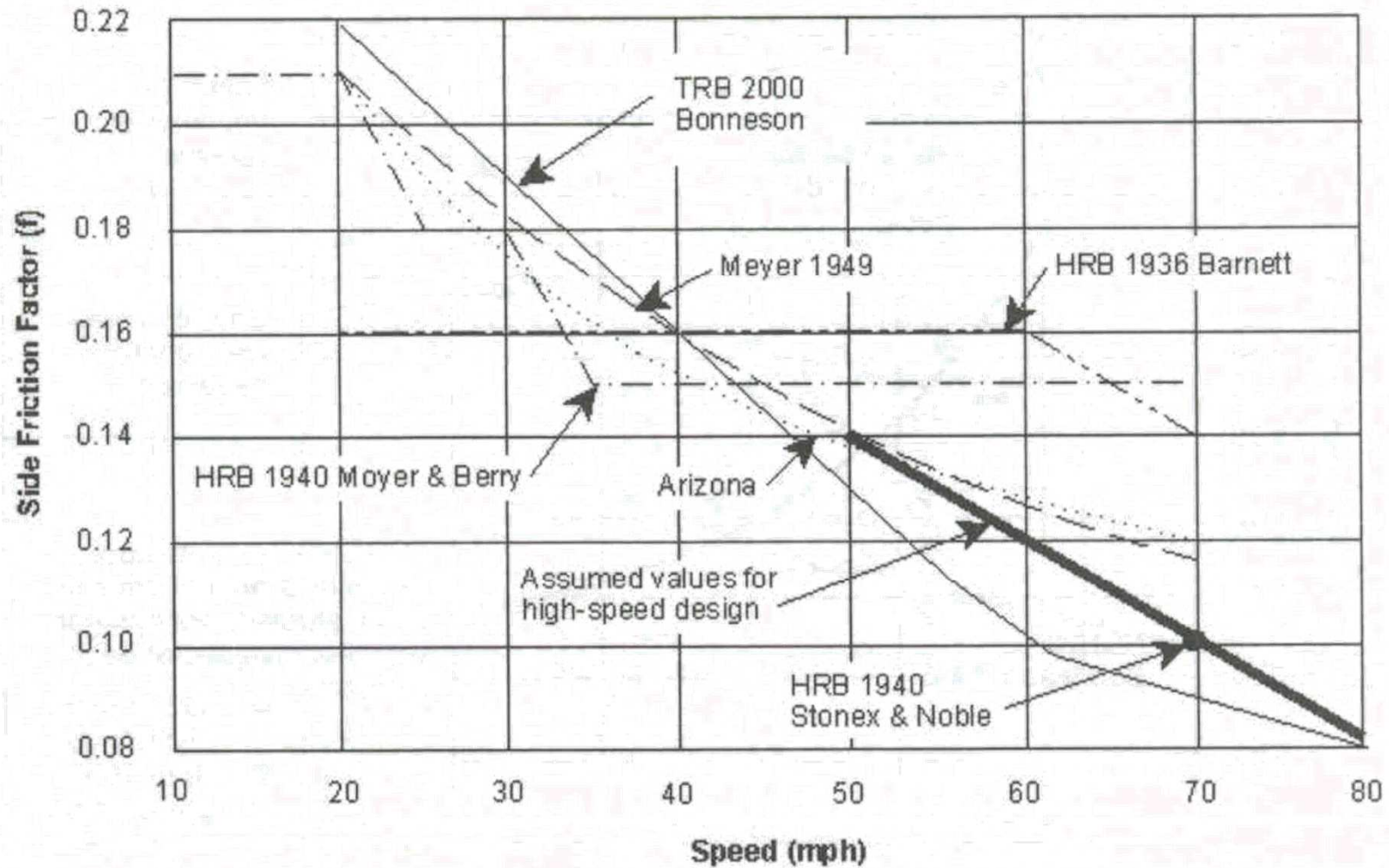
Centripetal Acceleration

- **Is counter-balanced by two factors:**
 - **Superelevation**
 - **Side Friction Factor**
- **Research has been conducted (dated) that has established limiting values for superelevation rate (e_{max}) and side friction demand (f_{max})**
- **Applying the limiting values results in the minimum curve radius for various design speeds**

Selection of e and f_s

- **Practical limits on superelevation (e)**
 - Climate
 - Constructability
 - Adjacent land use
- **Side friction factor (f_s) variations**
 - Vehicle speed
 - Pavement texture
 - Tire condition

Side Friction Factor 'f'



Minimum Radius Tables $e_{\max} = 4\%$

الحد الأدنى لنصف القطر (R_{\min}) عند (V_d) تساوي									الرفع الجانبي (e), %
100	90	80	70	60	50	40	30	20	
3250	2640	2170	1740	1310	951	679	371	163	1.5
2260	1830	1490	1180	877	632	441	237	102	2.0
1980	1590	1290	1020	749	534	363	187	75	2.2
1730	1390	1110	865	626	435	273	132	51	2.4
1510	1200	944	720	508	345	209	99	38	2.6
1320	1030	802	605	422	283	167	79	30	2.8
1150	893	690	516	356	236	137	64	24	3.0
1010	779	597	443	303	199	114	54	20	3.2
879	680	518	382	260	170	96	45	17	3.4
767	591	448	329	222	144	81	38	14	3.6
658	505	381	278	187	121	67	31	12	3.8
492	375	280	203	135	86	47	22	8	4.0

Minimum Radius Tables $e_{\max} = 6\%$

الحد الأدنى لنصف القطر (R_{\min}) عند (V_d) تساوي												الرفع الجانبي (e), %
130	120	110	100	90	80	70	60	50	40	30	20	
5240	4770	4060	3510	2880	2360	1910	1440	1050	738	421	194	1.5
3550	3510	2970	2560	2090	1710	1380	1030	750	525	299	138	2.0
3500	3160	2670	2300	1880	1530	1230	919	668	465	265	122	2.2
3190	2870	2420	2080	1700	1380	1110	825	599	415	236	109	2.4
2930	2630	2210	1890	1540	1260	1000	746	540	372	212	97	2.6
2700	2420	2020	1730	1410	1150	910	676	488	334	190	87	2.8
2510	2240	1870	1590	1290	1050	831	615	443	300	170	78	3.0
2330	2080	1730	1470	1190	959	761	561	402	269	152	70	3.2
2180	1940	1600	1360	1100	882	697	511	364	239	133	61	3.4
2050	1810	1490	1260	1020	813	640	465	329	206	113	51	3.6
1930	1700	1390	1170	939	749	586	422	294	177	96	42	3.8
1820	1590	1300	1090	870	690	535	380	261	155	82	36	4.0
1720	1500	1220	1010	806	635	488	343	234	136	72	31	4.2
1630	1410	1140	938	746	584	446	311	210	121	63	27	4.4
1540	1330	1070	873	692	538	408	382	190	108	56	24	4.6
1470	1260	997	812	641	496	374	258	172	97	50	21	4.8
1400	1190	933	755	594	457	343	235	156	88	45	19	5.0
1330	1120	871	701	549	421	315	214	142	79	40	17	5.2
1260	1060	810	648	506	386	287	195	128	71	36	15	5.4
1190	980	747	594	463	351	260	176	115	63	32	13	5.6
1110	900	679	537	416	315	232	156	102	56	28	11	5.8
951	756	560	437	336	252	182	123	79	43	21	8	6.0

Minimum Radius Tables $e_{\max} = 8\%$

الحد الأدنى لنصف القطر (R_{\min}) عند (V_d) تساوي												(e), %
130	120	110	100	90	80	70	60	50	40	30	20	
1220	1060	857	713	567	445	340	238	161	98	50	17	6.2
1180	1020	823	681	540	422	322	224	151	91	46	16	6.4
1140	982	789	651	514	400	304	210	141	85	43	15	6.6
1100	948	757	620	489	379	287	189	132	79	40	14	6.8
1070	914	724	591	464	358	270	185	123	73	37	13	7.0
1040	879	691	561	440	338	254	174	115	68	34	12	7.2
998	842	657	531	415	318	237	162	107	62	31	11	7.4
962	803	621	499	389	296	221	150	99	57	29	10	7.6
919	757	579	462	359	273	202	137	60	52	26	9	7.8
832	667	501	394	304	229	168	113	73	41	20	7	8.0

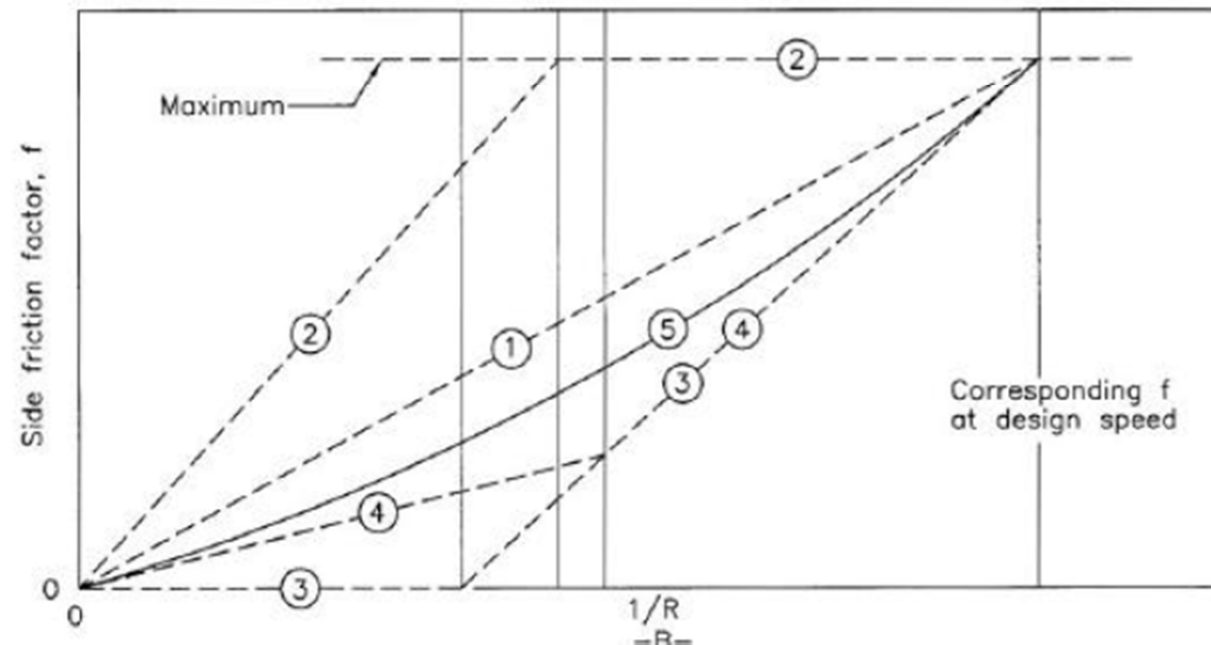
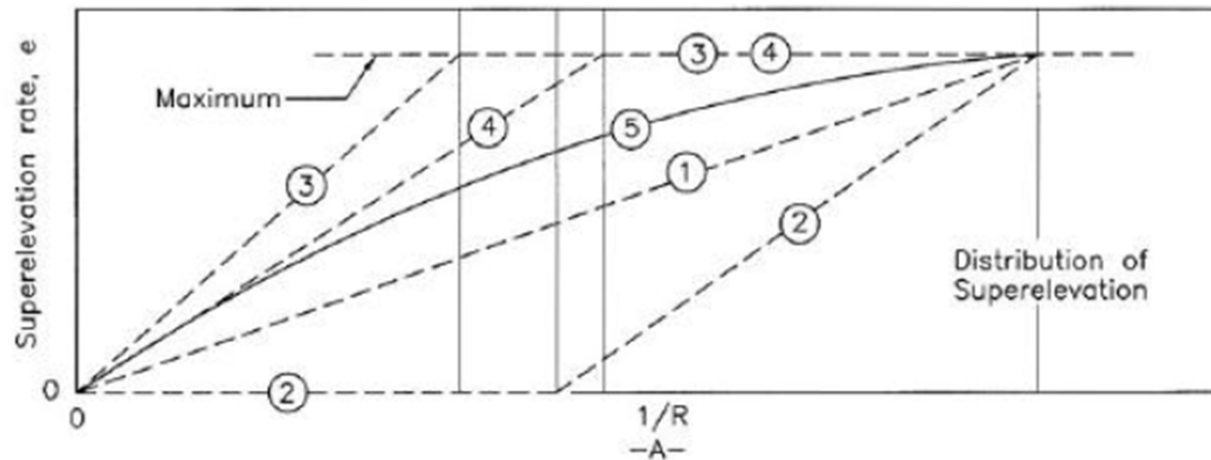
Minimum Radius Tables $e_{\max} = 10\%$

الحد الأدنى لنصف القطر (R_{\min}) عند (V_d) تساوي												(e)%
130	120	110	100	90	80	70	60	50	40	30	20	
1280	1140	941	795	640	538	404	298	210	138	77	31	6.2
1240	1100	907	766	616	516	387	285	200	130	72	28	6.4
1200	1060	876	738	593	496	372	273	191	121	67	26	6.6
1170	1030	846	712	571	476	357	261	181	114	62	24	6.8
1130	993	819	688	551	458	342	249	172	107	85	22	7.0
1100	963	792	664	532	441	329	238	164	101	55	21	7.2
1070	934	767	642	513	425	315	228	156	95	51	20	7.4
1040	907	743	621	496	409	303	218	148	90	48	18	7.6
1010	882	721	601	479	394	291	208	141	85	54	17	7.8
981	857	699	582	463	380	279	199	135	80	43	16	8.0
956	834	679	564	448	366	268	190	128	76	40	15	8.2
932	812	660	546	432	353	257	182	122	72	38	14	8.4
910	790	641	528	417	339	246	174	116	68	36	14	8.6
888	770	621	509	402	326	236	166	110	64	34	13	8.8
867	751	602	491	386	300	225	158	105	61	32	12	9.0
847	731	582	472	371	287	215	150	99	57	30	11	9.2
828	709	560	453	354	274	204	142	94	54	28	11	9.4
809	685	537	432	337	259	192	133	88	50	26	10	9.6
786	656	509	407	316	242	1789	124	81	46	24	9	9.8
739	597	454	358	277	210	154	105	68	38	19	7	10.0

Minimum Radius Tables $e_{\max} = 12\%$

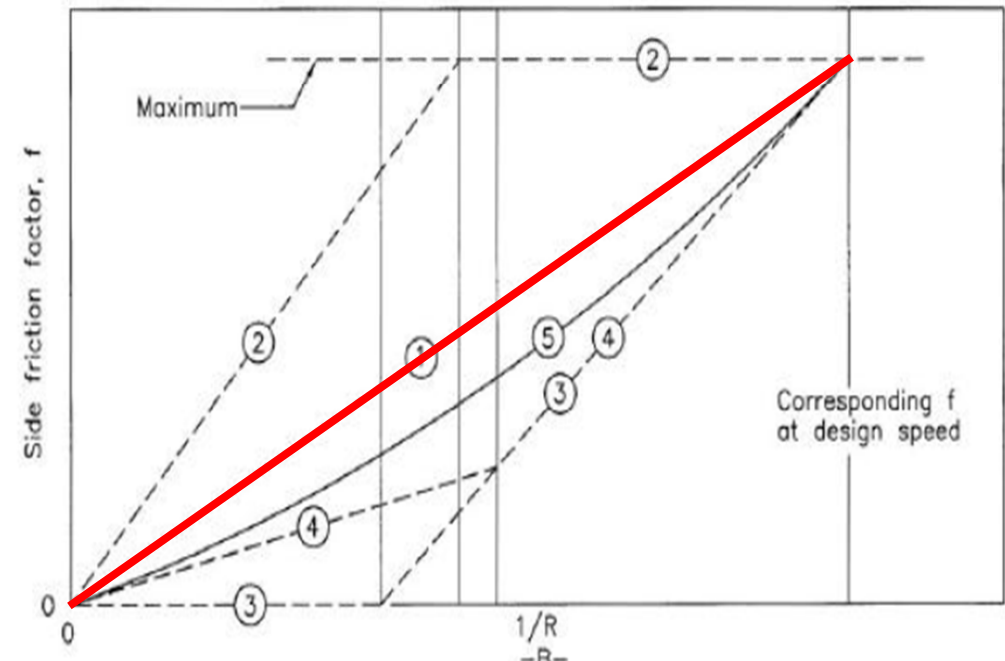
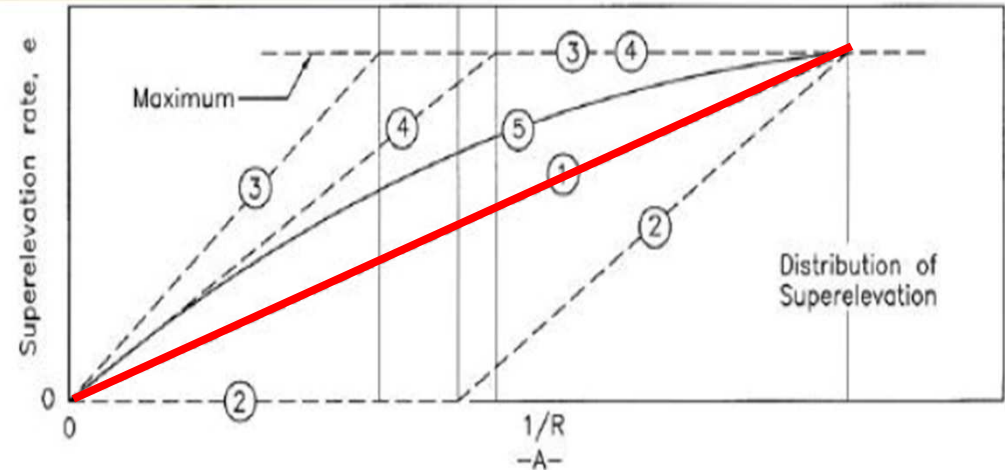
الحد الأدنى لنصف القطر (R_{\min}) عند (V_d) تساوي												(e), %
130	120	110	100	90	80	70	60	50	40	30	20	
1130	1010	834	708	572	462	364	270	192	128	71	32	7.2
1100	974	810	686	554	447	352	260	185	122	68	30	7.4
1070	947	786	666	537	433	340	251	178	117	65	29	7.6
1040	921	764	646	521	420	329	243	172	112	61	27	7.8
1020	897	743	628	506	402	319	235	165	107	58	26	8.0
989	874	723	610	491	395	306	227	159	102	55	24	8.2
965	852	704	593	477	383	399	219	154	97	52	23	8.4
942	831	686	577	464	372	390	212	148	93	50	22	8.6
921	811	668	562	451	361	381	205	142	88	47.3	20	8.8
900	792	652	547	439	351	273	198	137	85	45	19	9.0
880	774	636	533	428	341	264	191	132	81	43	18	9.2
861	756	621	520	416	332	256	185	127	77	41	18	9.4
843	739	606	507	406	323	249	179	123	74	39	17	9.6
826	723	592	494	395	314	241	173	118	71	37	16	9.8
809	708	579	482	385	305	234	167	114	68	36	15	10.0
793	693	566	471	375	296	226	161	110	65	34	14	10.2
778	679	553	459	365	288	219	155	105	62	33	14	10.4
763	665	541	448	355	279	212	150	101	59	31	13	10.6
749	652	529	436	345	270	204	144	97	57	30	12	10.8
735	639	516	423	335	261	197	139	93	54	28	12	11.0
722	626	503	411	324	252	189	133	89	51	27	11	11.2
709	613	488	397	312	242	182	127	85	49	25	11	11.4
697	598	472	397	300	232	173	120	80	46	24	10	11.6
685	579	453	382	285	219	163	113	75	43	22	9	11.8
665	540	414	364	255	194	143	98	64	36	18	7	12.0

Design Superelevation Rates - AASHTO



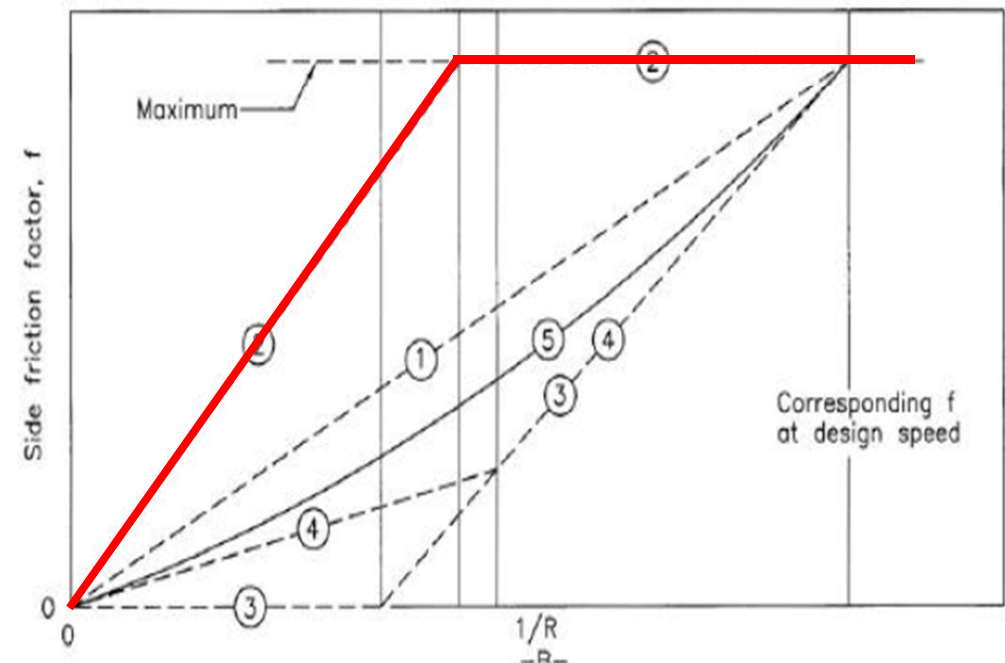
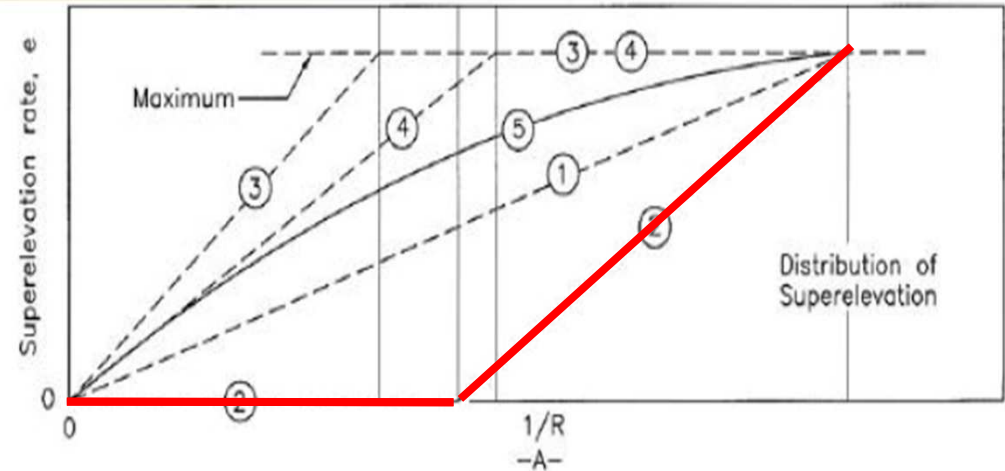
Method 1: e and f are directly proportional to $1/R$

- A straight line relation for e and f between $1/R=0$ and $1/R=1/R_{\min}$ for vehicles travelling at design or average running speed.
- Considerable merit and logic but simple.
- Appropriate if each vehicle travels at a constant speed on tangent and curve (intermediate degree or with minimum radius)
- Some drivers drive faster on tangents and flatter curves than on sharper curves.



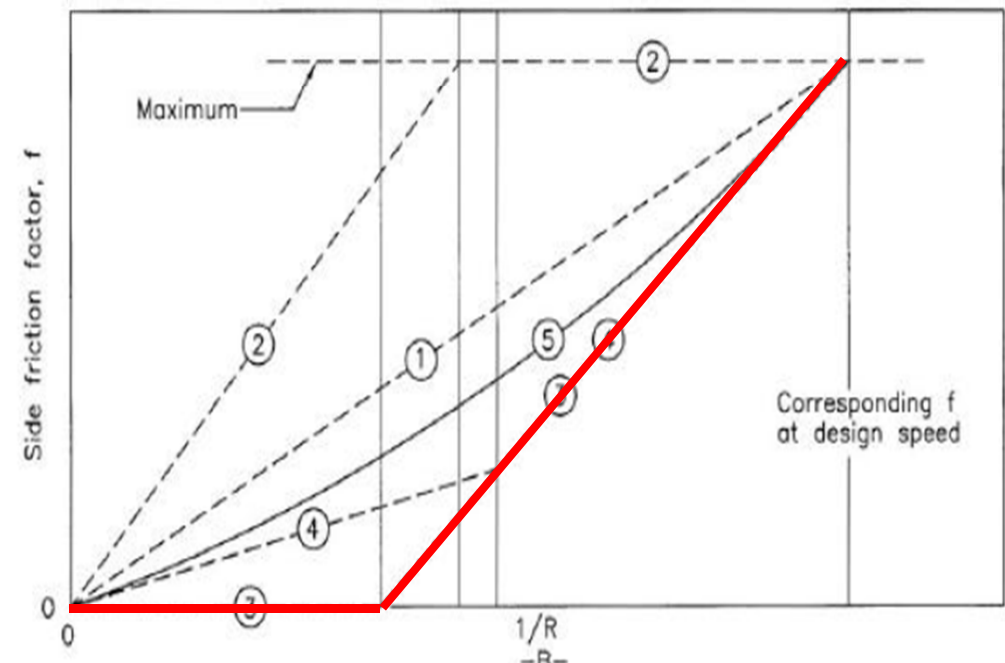
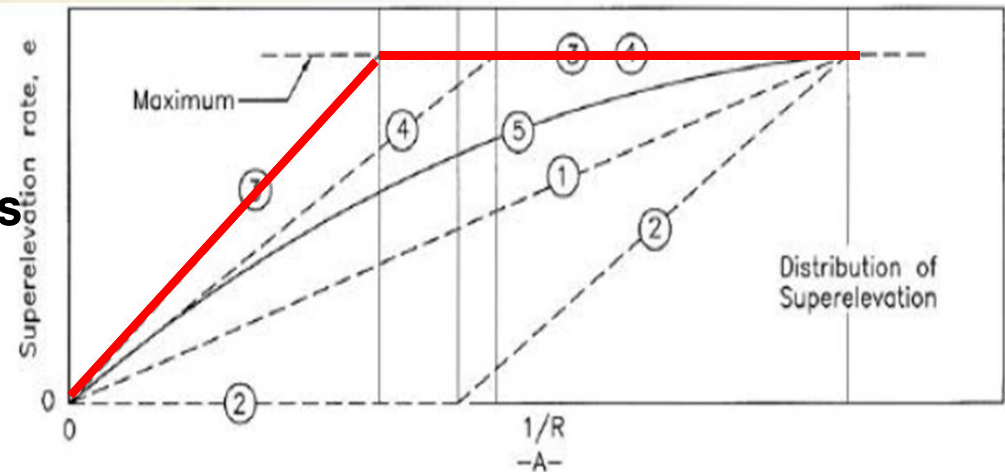
Method 2: f first reaches the maximum value then e starts increasing

- First f - then e are increased in inverse proportion to the radius of curvature.
- Possibility of no superelevation on flatter curves.
- Superelevation, when introduced, increase rapidly.
- Particularly advantageous on low speed urban streets where because of practical constraints, superelevation frequently cannot be provided



Method 3: e first reaches the maximum value then f starts increasing

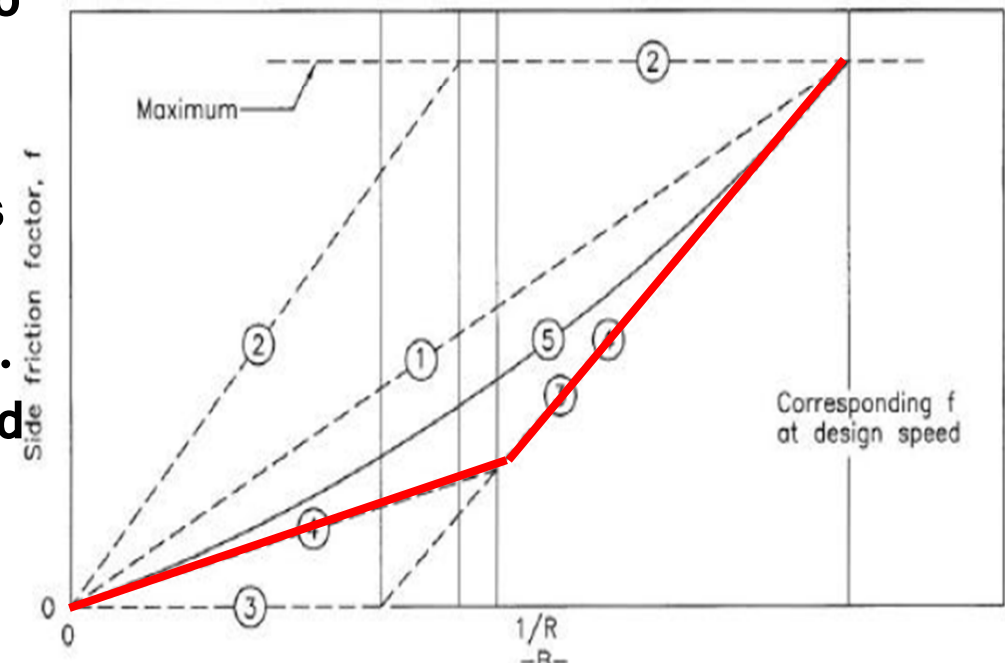
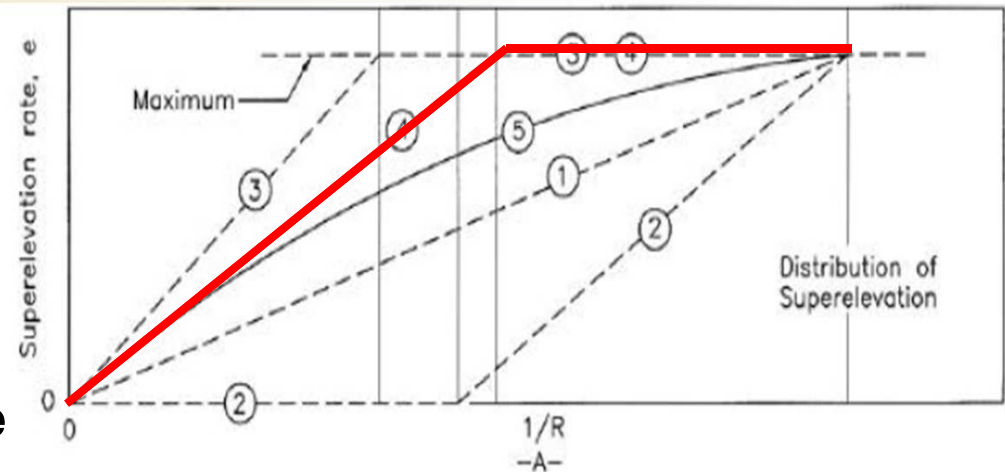
- e up to e_{\max} for vehicles travelling at design speed
- No side friction on flat curves ($e < e_{\max}$ at design speed)
- Beyond e_{\max} , f increase rapidly as curves become sharper.
- Results in negative f on flat curves for vehicles travelling at average running speed.
- Marked difference in f for different curves: Not logical and may result into erratic driving, either at design or average running speed.



Method 4: Same as method 3

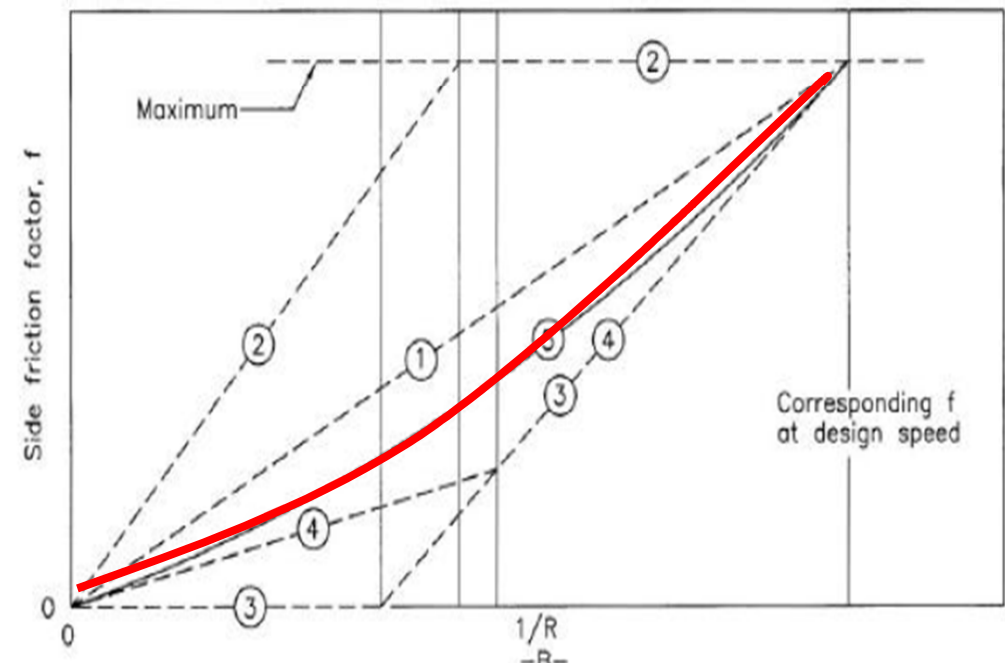
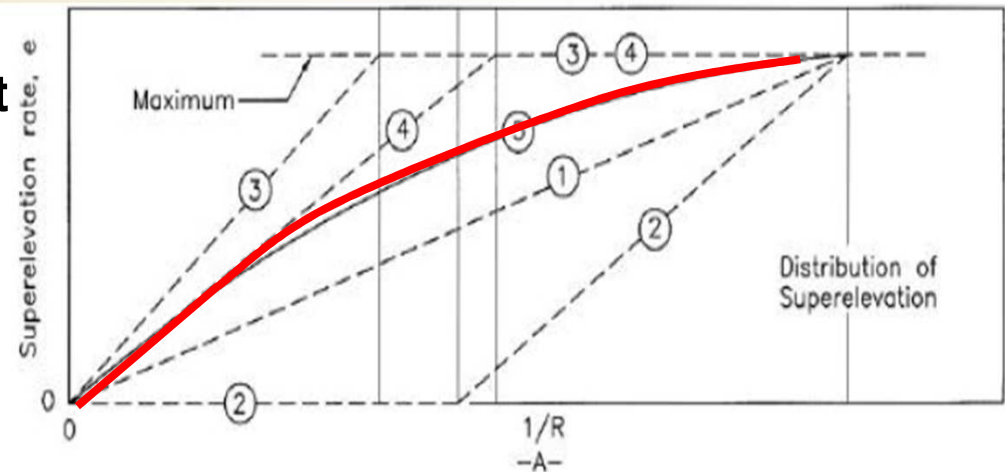
but this is based on average running speed instead of design speed

- Overcomes deficiency of method 3 by using e at speeds lower than design speed
- e_{\max} is reached near the middle of the curvature range - at average running speed no f is required up to this curvature.
- f , when introduced, increases rapidly and in direct proportion for sharper curves.
- Same disadvantage of method 3 but with smaller degree.



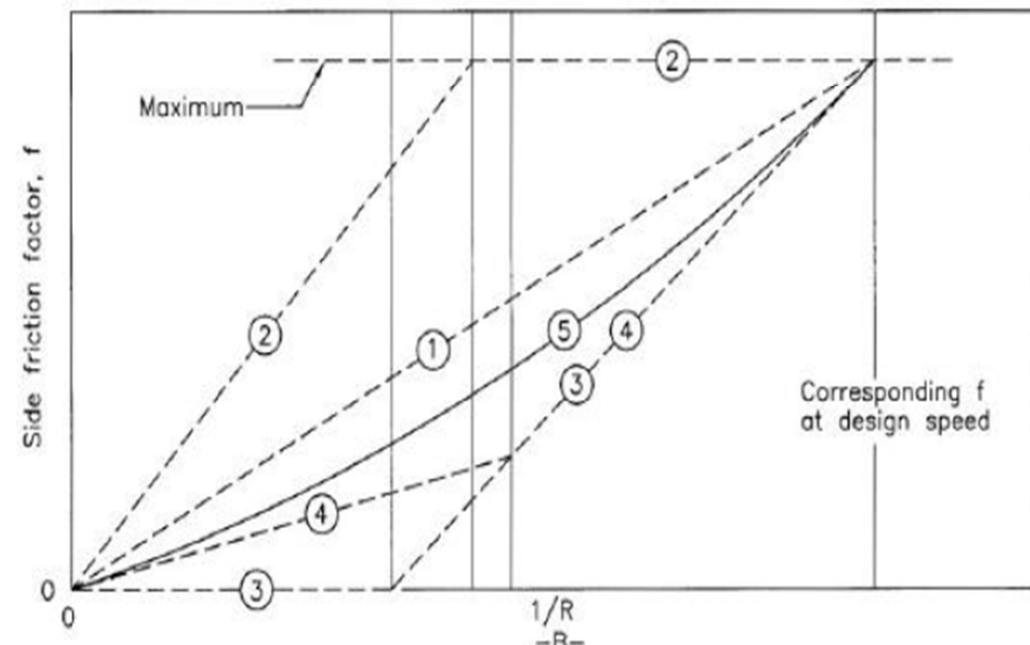
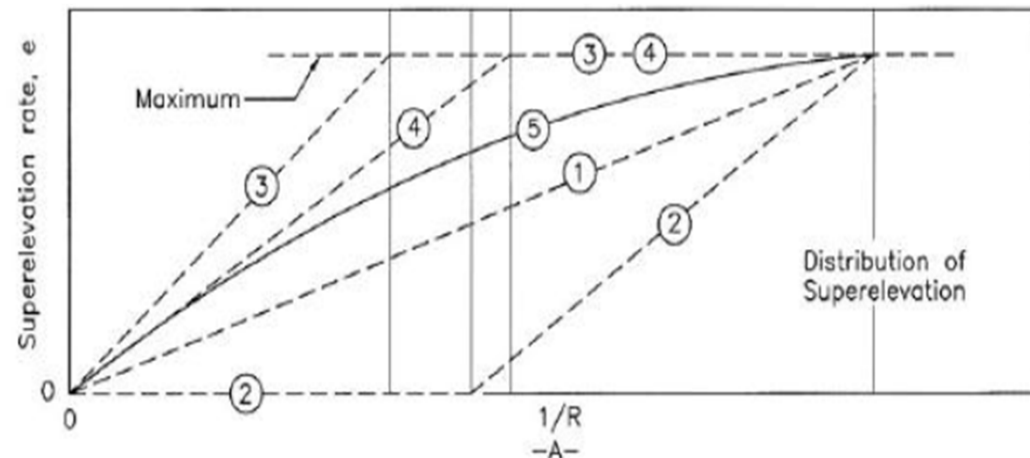
Method 5: e and f are in a curvilinear relation with $1/R$

- For overdriving (likely) on flat to intermediate curves: desirable to have 'e' similar to method 4.
- Very little risk for overdriving on such curves: 'e' adequate for average running speed and considerable 'f' for greater speed.

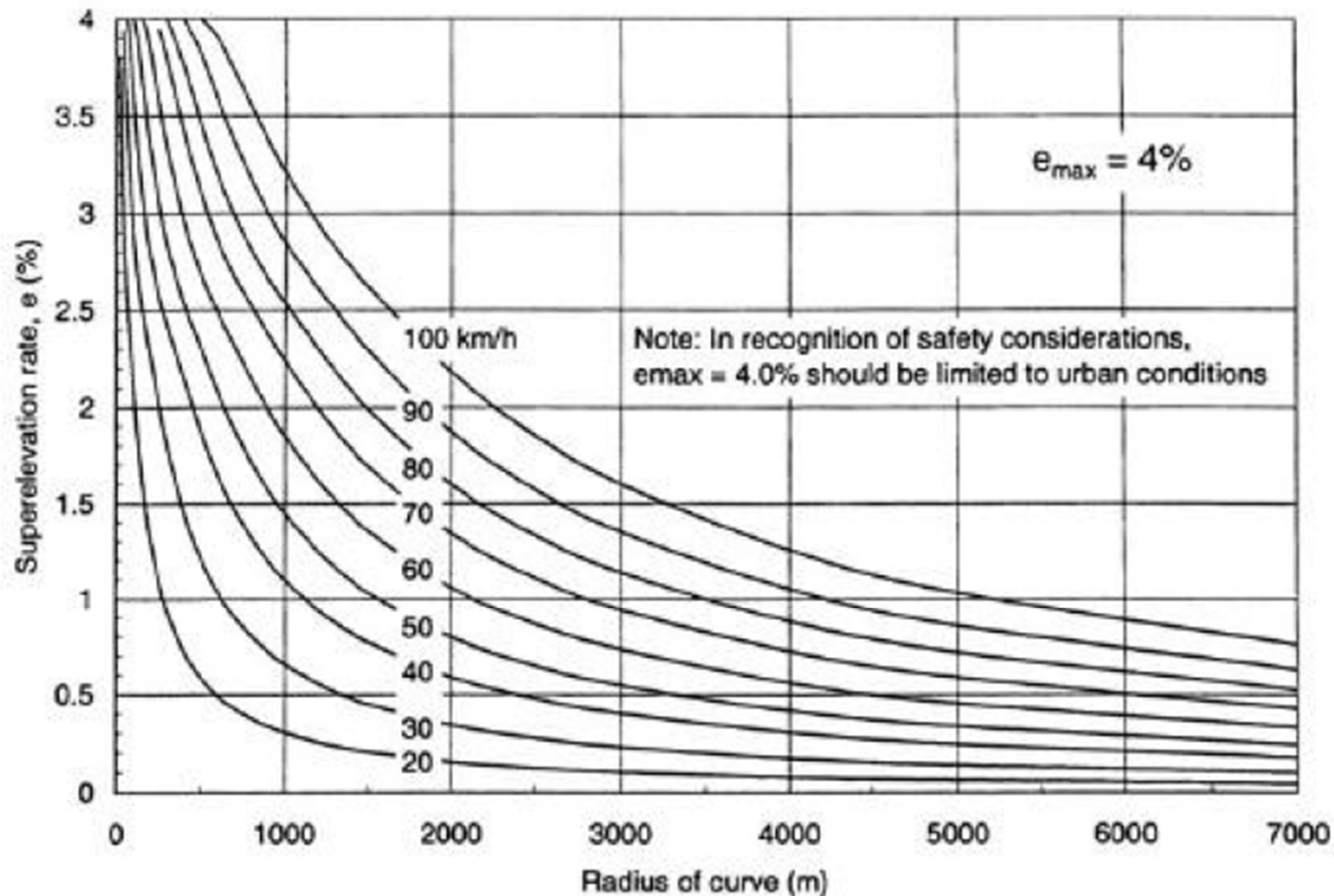


Design Superelevation Rates - AASHTO

- **Method 5:** distribution of 'e' and 'f' reasonably retaining the advantages of both method 1 and 4
- **Method 1:** is also desirable: a voids use of e_{\max} for a substantial part of the range of curve radii

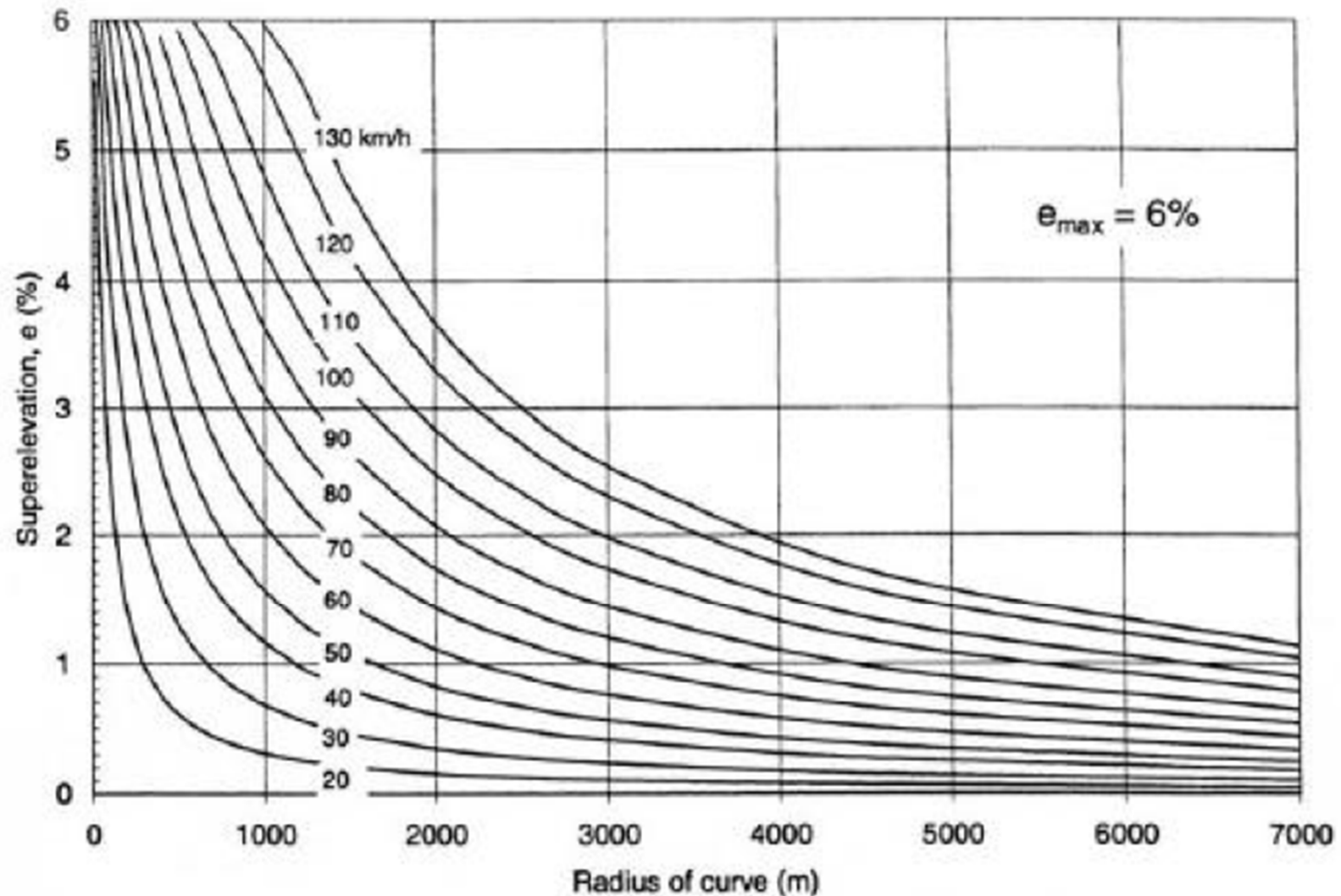


Design Superelevation Rates - AASHTO

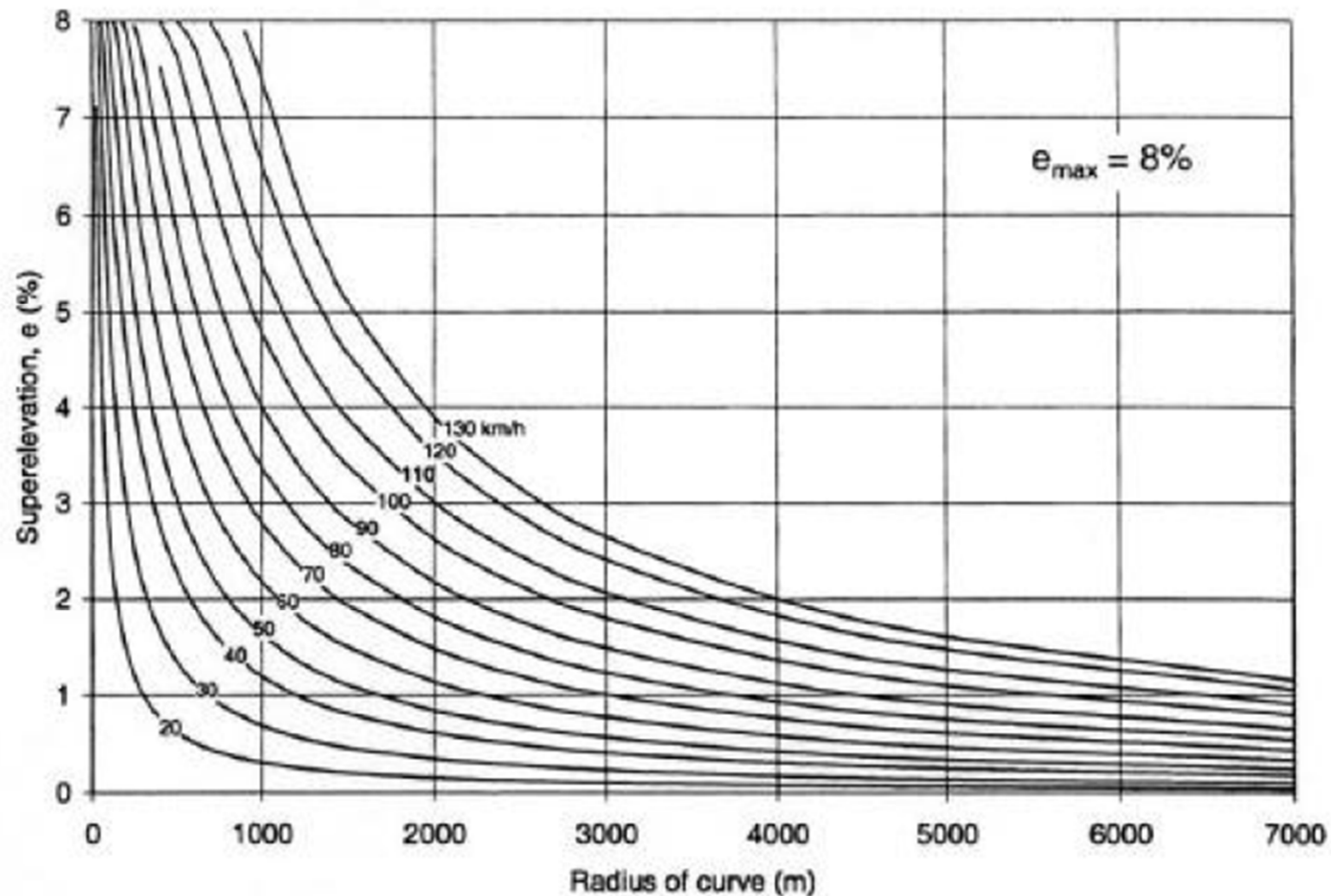


from AASHTO's *A Policy on Geometric Design of Highways and Streets* 2004

Design Superelevation Rates - AASHTO

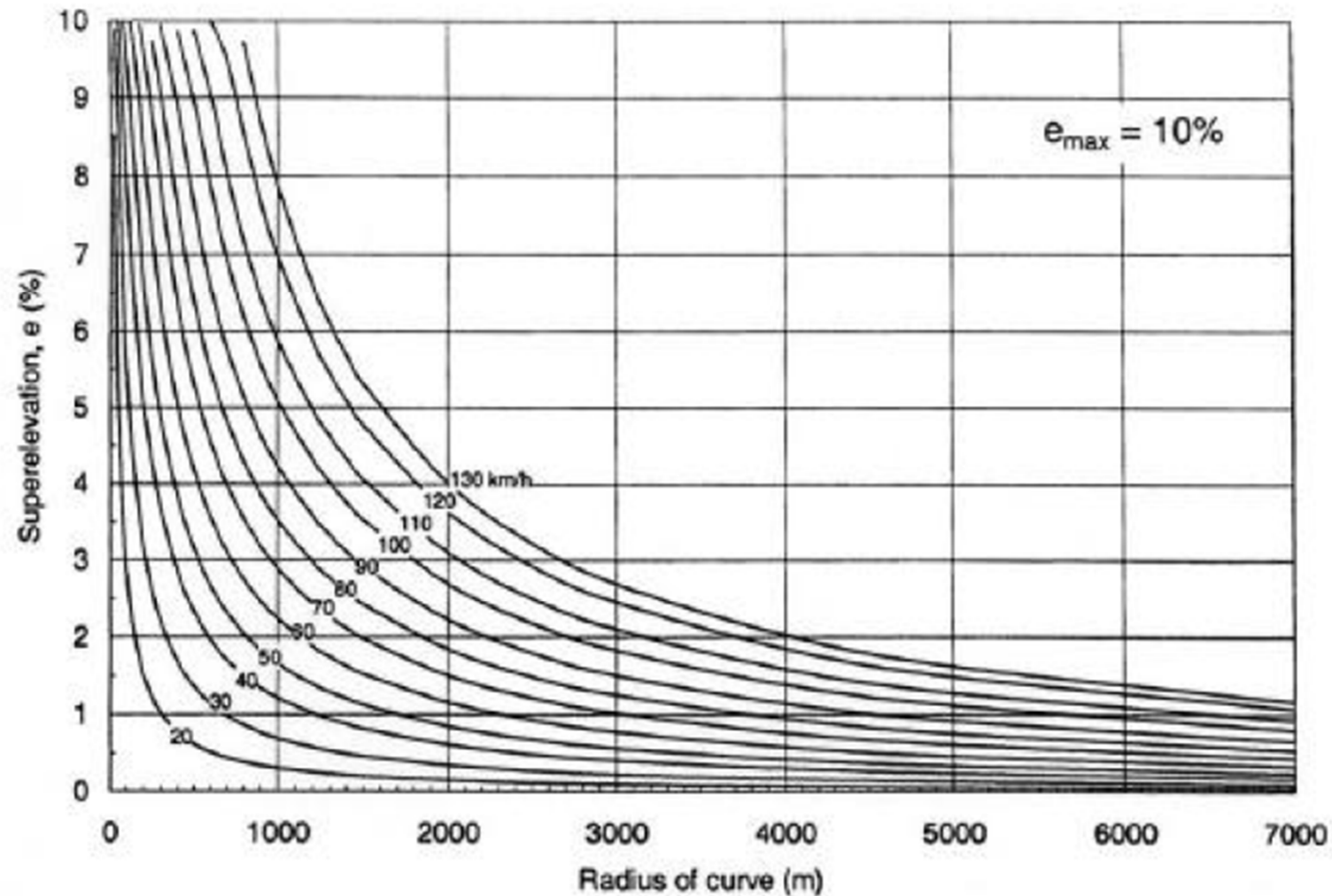


Design Superelevation Rates - WSDOT

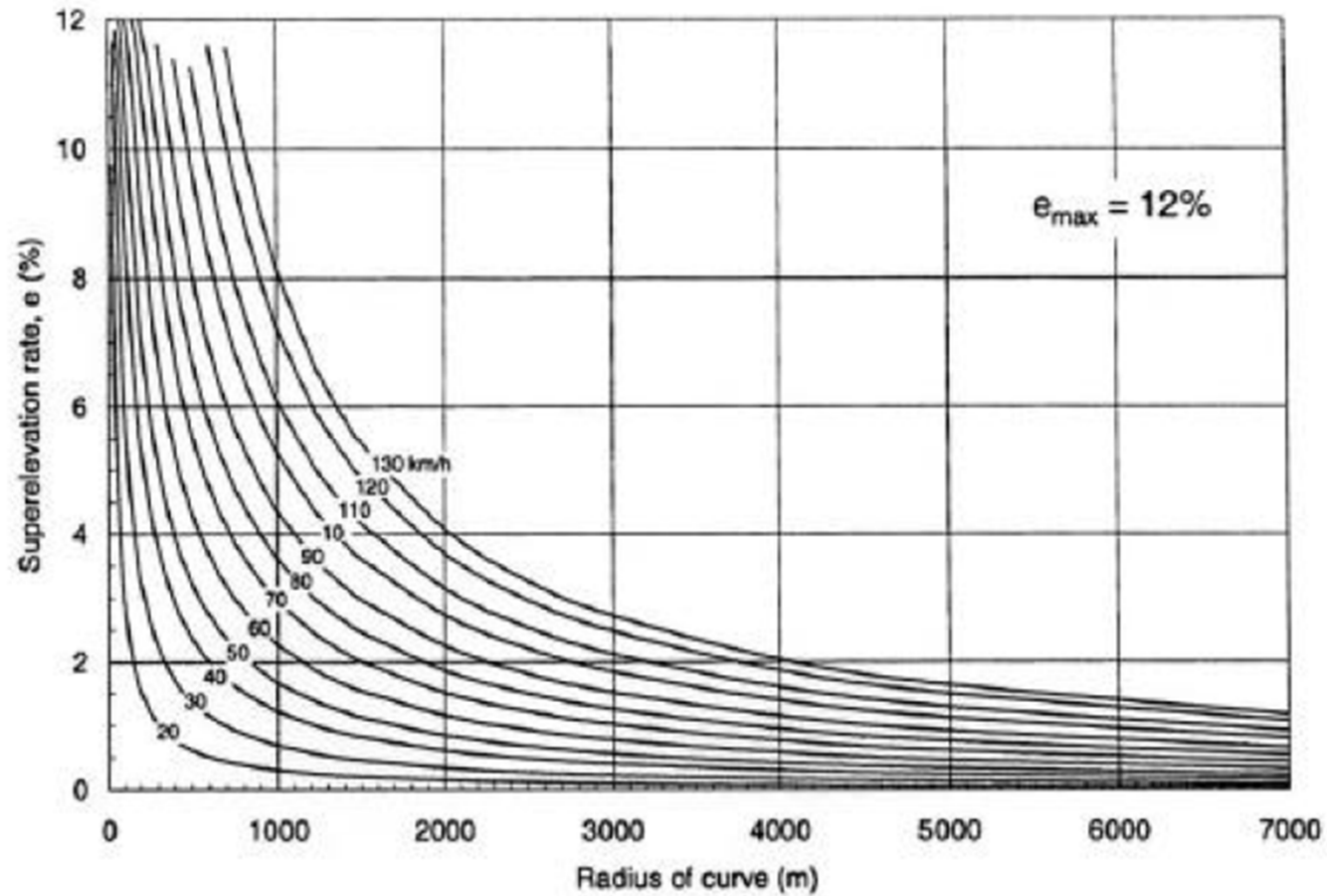


from the 2005 WSDOT *Design Manual*, M 22-01

Design Superelevation Rates - AASHTO



Design Superelevation Rates - AASHTO



Maximum Superelevation (AASHTO)

1. Depending on four factors:
 - Climate condition (frequently & amount of snow and ice)
 - Terrain condition (flat, rolling, mountainous)
 - Type of area (Urban, Rural)
 - Frequently of slow moving vehicles (affect high 'e')
2. No single e_{\max} is universally applicable
3. Desirable to use one e_{\max} (uniformity) within a region and similar climate for design consistency

Maximum Superelevation (AASHTO)

Five values: 4, 6, 8, 10, 12 percent

- ✓ 12%: Practical maximum value where snow and ice do not exist
- ✓ 10%: Highest superelevation rate for highways in common use
- ✓ 8%: Reasonable max value for low volume gravel surfaces roads - also max practical limit where snow and ice are factors
- ✓ 4 - 6%: Where traffic congestion and extensive marginal development acts to restrict top speeds.

(above 8% only in areas without snow and ice)

Example 5

A section of Blbs-Asher is being designed as a high-speed divided highway. The design speed is 100 kph. What is the minimum curve radius for safe vehicle operation?

الحمد لله
أي سؤال؟